DESIGN LOADS PER MASSACHUSETTS STATE BUILDING CODE

LIVE LOADS

GROUND SNOW LOAD:	40 PSF
SLEEPING AREAS:	30 PSF
ALL OTHER AREAS EXCEPT DECKS AND BALCONIES	40 PSF
EXTERIOR BALCONIES AND DECKS	60 PSF
COMMON AREAS	100 PSF

WIND LOADS

MASSACHUSETTS STATE BUILDING CODE 128 MPH, EXPOSURE B

DEAD LOAD

WEIGHTS OF MATERIALS AND CONSTRUCTION

GENERAL CONDITIONS

1. G. C. MUST BUILD EXACTLY WHAT IS SHOWN ON STRUCTURAL DRAWINGS. ANY PROPOSED DEPARTURES FROM WHAT IS INDICATED MUST BE REVIEWED WITH THE ENGINEER PRIOR TO CONSTRUCTION. ALL UNAUTHORIZED CHANGES TO THE APPROVED DRAWINGS MUST BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE.
2. THE CONTRACTOR SHALL CAREFULLY VERIFY ALL DIMENSIONS AND CONDITIONS SHOWN ON DRAWINGS PRIOR TO COMMENCEMENT OF THE WORK, AND SHALL NOTIFY THE ENGINEER IMMEDIATELY OF ANY DISCREPANCIES BETWEEN ENGINEERING AND ARCHITECTURAL DOCUMENTS.
3. THE CONTRACTOR IS RESPONSIBLE FOR ALL MEANS AND METHODS OF TEMPORARY SHORING, BRACING, OR OTHERWISE PROTECTING ANY PORTION OF THE STRUCTURE, SITE AND UTILITIES FROM DAMAGE DURING CONSTRUCTION. THE ENGINEER IS SPECIFYING THE FINISHED CONDITION ONLY, WITHOUT ASSUMING KNOWLEDGE NOR RESPONSIBILITY FOR HOW THE CONTRACTOR WILL ACHIEVE THIS RESULT.
4. FOR RENOVATION WORK STRUCTURAL DRAWINGS PRODUCED WITH ASSUMPTIONS MADE REGARDING EXISTING CONDITIONS. IF CONTRACTOR FINDS EXISTING CONDITIONS NOT AS ASSUMED CONTACT ENGINEER IMMEDIATELY. REVISIONS TO THE STRUCTURAL FRAMING MAY BE REQUIRED.
5. FOR EXACT LOCATIONS OF FLOOR AND ROOF OPENINGS, POSTS, ETC., SEE ARCHITECTURAL DRAWINGS.

FOUNDATIONS

1. WHERE FOUNDATIONS ARE EXISTING, DESIGN HAS BEEN COMPLETED ASSUMING FOUNDATIONS ARE SUITABLE TO SUPPORT PROPOSED RENOVATION. CONTRACTOR RESPONSIBLE FOR VERIFYING THAT THE EXISTING FOUNDATION CONFORMS TO BUILDING CODE REQUIREMENTS AND REPORT FOOTING CONDITIONS TO ENGINEER FOR VERIFICATION.
2. EXCAVATE TO LINES AND GRADES REQUIRED TO PROPERLY INSTALL THE FOUNDATIONS ON INORGANIC, UNDISTURBED SOIL OR CONTROLLED STRUCTURAL BACKFILL AS REQUIRED BY THE ARCHITECT. ALL EXCAVATIONS SHALL BE DRY BEFORE PLACING ANY CONCRETE.
3. EXTERIOR FOOTINGS SHALL BE PLACED ON APPROVED SOIL AT A MINIMUM DEPTH OF 4 FEET, OR AS MODIFIED BY THE STRUCTURAL ENGINEER, BELOW THE LOWEST ADJACENT GROUND EXPOSED TO FREEZING. ANY ADJUSTMENT OF FOOTING ELEVATIONS DUE TO FIELD CONDITIONS MUST HAVE THE APPROVAL OF THE ARCHITECT.
4. SOIL BEARING CAPACITY: FOOTINGS MUST BE PLACED ON SOIL WITH A MINIMUM BEARING CAPACITY OF 4000 POUNDS PER SQUARE FOOT.
5. BACKFILL BELOW FOOTINGS AND SLABS SHALL BE MADE WITH APPROVED GRANULAR MATERIALS PLACED IN 6" LAYERS. LAYERS SHALL BE COMPACTED TO 96% DENSITY AT OPTIMUM MOISTURE CONTENT, AS DEFINED BY ASTM D1557.
6. BACKFILLING AGAINST WALLS OR PIERS MAY ONLY BE DONE AFTER WALLS OR PIERS ARE BRACED TO PREVENT MOVEMENT. FOR WOOD FRAMED RESIDENTIAL CONSTRUCTION, NO BACKFILLING OF WALLS MAY TAKE PLACE UNTIL THE FIRST FLOOR DECK HAS BEEN FRAMED AND SHEATHED, UNLESS WRITTEN APPROVAL IS GIVEN BY THE ARCHITECT OR ENGINEER.
7. PROVIDE FOUNDATION DRAINAGE, WATERPROOFING/DAMP-PROOFING, AND FOUNDATION WALL INSULATION AS INDICATED ON THE ARCHITECTURAL DRAWINGS.

CONCRETE

1. ALL CONCRETE WORK SHALL BE PERFORMED IN CONFORMANCE WITH THE LATEST EDITION OF ACI-318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE".
2. CONCRETE SHALL ACHIEVE A MINIMUM 28 DAY DESIGN STRENGTH AS FOLLOWS:FOOTINGS, WALLS, INTERIOR SLABS-ON-GRADE, AND OTHER CONCRETE NOT OTHERWISE SPECIFIED – 3000 PSI. EXTERIOR SLABS EXPOSED TO WEATHER – 4000 PSI.
3. SLUMP AT THE POINT OF DISCHARGE FROM THE READY-MIX TRUCK SHALL BE 3-5".
4. REINFORCING STEEL: TYPICAL – ASTM A615, GRADE 60. FIELD BENT – ASTM A615, GRADE 40 WELDED WIRE FABRIC – ASTM A185.

HANGER SELECTION TABLE

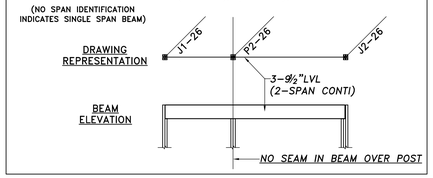
MEMBER \ QT	1	2	3
2X8	LUS28	LUS28-2	LUS28-3
2X10	LUS210	LUS210-2	LUS210-3
2X12	LUS210	LUS210-2	LUS210-3
9½"LVL	HU9	HU410	HU610
11⅞"LVL	HU11	HU412	HU612
14"LVL	HU14	HU416	HU616
2⅝"FLG I-JOIST	IUS 2.37		
2½"FLG I-JOIST	IUS 2.56		
3½"FLG I-JOIST	IUS 3.56		

NOTE:
1. USE HANGERS ABOVE FOR PROPOSED STRUCTURE UNLESS OTHERWISE NOTED ON FRAMING PLANS.
2. INSTALL ALL HANGERS WITH MAXIMUM NUMBER OF FASTENERS.

MULTIPLE SPAN BEAM LEGEND

BEAMS IDENTIFIED AS [(MULTI)-SPAN] CONTINUOUS

EXAMPLE: (2-SPAN), (3-SPAN), ETC.



ROUGH CARPENTRY

1. ALL ROUGH CARPENTRY WORK SHALL BE EXECUTED IN CONFORMANCE WITH THE LATEST EDITION OF THE MASSACHUSETTS BUILDING CODE (MBC) AND THE INTERNATIONAL BUILDING CODE (IBC).
2. REFER THE MBC AND IBC FOR FRAMING COMPONENTS NOT SPECIFIED IN PLANS AND SECTIONS. NOTIFY THE ENGINEER OF ANY COMPONENT NOT DEFINED IN EITHER THE MBC AND IBC OR IN THESE DRAWINGS.
3. 3. REFER TO IBC FASTENER SCHEDULE FOR STRUCTURAL MEMBERS TABLE 2304.9.1 FOR CONNECTION FASTENING NOT IDENTIFIED IN THESE PLANS OR DETAILS.
4. ENGINEER MAKES NO CLAIMS TOWARDS EXISTING CONDITIONS.
5. WHEN NOT OTHERWISE IDENTIFIED, ALL WOOD BEAMS, JOISTS, RAFTERS, HEADERS, STRINGERS, PLATES, AND SILLS SHALL BE SPRUCE PINE FIR #2 OR BETTER, WITH A MINIMUM Fb = 875 PSI (SINGLE USE) AND Fb = 1000 PSI (REPETITIVE USE), AND E SHALL BE 1,4000,000 PSI OR BETTER.
6. WOOD STUDS MAY BE EASTERN HEMLOCK, EASTERN SPRUCE, OR HEM-FIR, GRADED "STUD" GRADE, #2 OR BETTER.
7. LVL BEAMS, AS NOTED ON PLANS, SHALL HAVE A MINIMUM Fb = 3100 PSI, E = 2,000,000 PSI, AND Fv = 285 PSI. LVL BEAMS SHALL BE "VERSALAM" BY BOISE CASCADE. NO SUBSTITUTIONS WILL BE ACCEPTED, UNLESS THE ENGINEER SPECIFICALLY APPROVES ANOTHER PRODUCT SUBMITTED BY THE CONTRACTOR.
8. WOOD "I" BEAMS SHALL BE BY BOISE CASCADE. NO SUBSTITUTIONS WILL BE ACCEPTED, UNLESS THE ENGINEER SPECIFICALLY APPROVES ANOTHER PRODUCT SUBMITTED BY THE CONTRACTOR. MANUFACTURER'S RECOMMENDATIONS FOR BEARING, REINFORCING, CUTS, CANTILEVERS, FASTENING, ETC., SHALL BE STRICTLY ADHERED TO.
9. ENGINEERED WOOD POSTS (VERSA COLUMNS), AS NOTED ON PLANS, SHALL BE VERSA-LAM 1.7 2650.
10. PLYWOOD WALL SHEATHING, ROOF SHEATHING, AND SUBFLOORING SHALL BE APA GRADE, TRADEMARKED C-D INTERIOR WITH EXTERIOR GLUE. SUBFLOORING SHALL BE 3/4" THICK TONGUE AND GROOVE, AND SHALL BE GLUED TO FLOOR JOISTS WITH AN APPROVED ADHESIVE PRIOR TO NAILING. ROOF SHEATHING SHALL BE 1/2" THICK AND WALL SHEATHING SHALL BE 1/2" THICK.
11. ALL WOOD HAVING DIRECT CONTACT WITH CONCRETE OR MASONRY, AND WHEREVER WOOD IS WITHIN 8" OF FINISHED GRADE OR PART OF OPEN DECK CONSTRUCTION, SHALL BE PRESSURE TREATED.
12. ALL METAL CONNECTORS INCLUDING JOIST AND BEAM HANGERS AND COLUMN CAP AND BASES SHALL BE BY SIMPSON STRONG-TIE CORP. THE CONTRACTOR SHALL STRICTLY ADHERE TO MANUFACTURER'S FASTENING REQUIREMENTS. CONTRACTOR TO VERIFY ALL CONNECTOR SIZES TO FRAMING ELEMENTS BEFORE ORDERING.
13. UNLESS DETAILED OR SPECIFIED OTHERWISE ON THE PLANS, HEADERS AND BEAMS SHALL BE SUPPORTED BY AT LEAST ONE JACK STUD AND ONE KING STUD.
14. FOR WOOD JOIST SPANS UP TO 14 FEET, PROVIDE A SINGLE ROW OF FULL DEPTH BLOCKING BETWEEN JOISTS AT MIDSPAN. FOR SPANS EXCEEDING 14 FEET, PROVIDE TWO ROWS OF FULL DEPTH BLOCKING BETWEEN JOISTS AT THIRD POINTS OF THE SPAN.
15. GABLE-END WALL STUDS IN CATHEDRAL, PARTIAL CATHEDRAL, OR HIGH CEILING SPACES SHALL SPAN UNINTERRUPTED FROM THE FLOOR PLATE TO THE UNDERSIDE OF THE ROOF RAFTERS. THEY SHOULD NOT BE INTERRUPTED BY ANY HORIZONTAL PLATES OR BEAMS, UNLESS NOTED OTHERWISE ON THE DRAWINGS.
16. MEMBERS WITHIN BUILT-UP BEAMS, WHETHER MADE OF SAWN OR ENGINEERED LUMBER, SHALL ONLY BE SPLICED OVER SUPPORTS.
17. PROVIDE SIMPSON H1 OR H2.5 HURRICANE TIES BETWEEN EACH RAFTER BOTTOM AND ITS BEARING POINT.
18. CONTRACTOR SHALL CAREFULLY COORDINATE THE WORK OF ALL TRADES TO MINIMIZE THE NEED FOR CUT, BORED OR NOTCHED IN FRAMING LUMBER. STRUCTURAL FLOOR MEMBERS SHALL NOT BE CUT, BORED OR NOTCHED IN EXCESS OF THE LIMITATIONS SPECIFIED IN THE BUILDING CODE WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.
19. AT WOOD POSTS LANDING ON FLOOR DECK, PROVIDE SOLID VERTICAL WOOD BLOCKING WITHIN DECK SANDWICH TO LINK UPPER POST WITH LOWER SUPPORT. BLOCKING TO MATCH UPPER POST SIZE.
20. SET LVL BEAMS THAT FRAME FLUSH WITH DIMENSIONED LUMBER JOISTS 3/8" BELOW THE TOP OF JOISTS TO ALLOW FOR JOIST SHRINKAGE. WHERE BEARING WALLS OR POSTS LAND ON THESE BEAMS, INFILL GAP WITH 3/8" PLYWOOD FOR SOLID BEARING.
21. BEAMS COMPRISED OF 3 LVLS OR MORE SHALL BE BOLTED TOGETHER WITH A MINIMUM OF 2-1/2"ø BOLTS AT 16" ON CENTER OR 3-¼"ø DIAMETER SELF TAPPING LAG SCREWS AT 16" ON CENTER, ALTERNATING INSERTION SIDES, FOLLOW MANUF. SPECS, UNLESS NOTED OTHERWISE ON DRAWING.
22. IN ADDITION TO THE FLOOR JOIST SHOWN IN THE PLANS, CONTRACTOR SHALL INSTALL DOUBLE JOISTS UNDER ALL PARTITIONS WALLS RUNNING PARALLEL TO THE DIRECTION OF FRAMING.
23. MINIMUM BEAM BEARING TO BE 3 INCHES UNLESS NOTED OTHERWISE ON PLAN.
24. BEARING WALL SCHEDULE
    -ALL EXTERIOR WALLS:
    2x6@16"OC WITH 2 ROWS OF HORIZONTAL BLOCKING AT ⅓ POINTS
    -1ST FLOOR INTERIOR BEARING WALLS:
    2x6@16"OC WITH 2 ROWS OF HORIZONTAL BLOCKING AT ⅓ POINTS
    -2ND & 3RD FLOOR INTERIOR BEARING WALLS:
    2x6@16"OC WITH 1 ROW OF HORIZONTAL BLOCKING AT MID-HEIGHT OF WALL

STRUCTURAL STEEL

1. STRUCTURAL STEEL WORK SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION: "SPECIFICATION FOR STRUCTURAL STEEL FOR BUILDINGS", LATEST EDITION.
2. STEEL WIDE FLANGE BEAMS SHALL CONFORM TO ASTM A992, WITH A MINIMUM YIELD STRENGTH OF 50 KSI. PLATES, ANGLES, CHANNELS, AND MISC. FABRICATED HARDWARE SHALL CONFORM TO ASTM A36, WITH A MINIMUM YIELD STRENGTH OF 36 KSI. RECTANGULAR STEEL TUBING SHALL CONFORM TO ASTM A500, GRADE B, WITH A MINIMUM YIELD STRENGTH OF 46 KSI.
3. ALL STEEL TO STEEL FIELD CONNECTIONS SHALL BE MADE BY HIGH STRENGTH BOLTING WITH ASTM A325 BOLTS OR WELDING WITH E70 XX ELECTRODES. STEEL TO CONCRETE AND STEEL TO WOOD FIELD CONNECTIONS MAY BE MADE WITH ASTM A 307 BOLTS.
4. STEEL SHALL BE SHOP-PAINTED WITH A MODIFIED ALKYD PRIMER UNLESS OTHERWISE NOTED.
5. NO CUTTING OF OR OPENINGS THROUGH STEEL WILL BE PERMITTED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.
6. CONTRACTOR TO SUBMIT SHOP DRAWING TO ARCHITECT AND ENGINEER FOR APPROVAL.

ABBREVIATIONS:

ADD'L – ADDITIONAL	HORIZ – HORIZONTAL
BLKG – BLOCKING	HDR – HEADER
BM – BEAM	JST – JOIST
BTM – BOTTOM	LVL – LAMINATED VENEER LUMBER
BRG – BEARING	LW – LONG WAY
BTWN – BETWEEN	MAX – MAXIMUM
BW –BEARING WALL	MIN – MINIMUM
CLG – CEILING	MFR – MANUFACTURER
COL – COLUMN	NTS – NOT TO SCALE
CONC – CONCRETE	OC – ON CENTER
CONN – CONNECTION	PL – PLATE
CONT – CONTINUOUS	PT – PRESSURE TREATED
DIAG – DIAGONAL	REQ – REQUIRED
DN – DOWN	SPEC – SPECIFICATION
EQSP – EQUAL SPACES	SW – SHORT WAY
ES – EACH SIDE	TYP – TYPICAL
EW – EACH WAY	UON – UNLESS OTHERWISE NOTED
FDN – FOUNDATION	VERT – VERTICAL
FIN – FINISH	W/ – WITH
FLG – FLANGE	
FTG – FOOTING	
TFN – TOP FLANGE NAILER	

REINFORCED MASONRY:

1. MASONRY CONSTRUCTION SHALL CONFORM TO "BUILDING CODE REQUIREMENTS AND FOR CONCRETE MASONRY CONSTRUCTION (ACI 530.05ASCE 5-05/TMS 602-05) AND SPECIFICATIONS FOR MASONRY STRUCTURES AND RELATED COMMENTARIES (ACI 530/530.1-05/ASCE 605/TMS 602-05).
2. MASONRY UNITS SHALL CONFORM TO ASTM C55 OR ASTM C90 AND ARE SAMPLED AND TESTED IN ACCORDANCE WITH ASTM C140. F'm = 1500PSI.
3. THICKNESS OF BED JOINTS DOES NOT EXCEED ⅝"
4. MORTAR FOR BLOCK WALL CONSTRUCTION SHALL BE TYPE M OR S CONFORMING TO ASTM C270.
5. GROUT FOR PIERS AND BLOCK WALLS SHALL CONFORM TO ASTM C476 WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 psi DETERMINED IN ACCORDANCE WITH THE PROVISIONS OF ASTM C1019.
6. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, EXCEPT BARS TO BE WELDED SHALL CONFORM TO ASTM A706.
7. WIRE FOR JOINT REINFORCING SHALL CONFIRM TO ASTM A82, YIELD POINT = 70 ksi (MIN).
8. UNLESS NOTED OTHERWISE ON PLANS, PROVIDE THE FOLLOWING MINIMUM REINFORCEMENT:
9. #5 @ 32" OC VERTICAL AND #9 GA. LADDER OR TRUSS TYPE @ 16"OC HORIZONTAL.
10. PROVIDE BOND BEAMS WITH 1-#5 CONTINUOUS, AT THE TOP OF FOUNDATION WALLS AND THE TOP OF PARAPETS, AT EACH FLOOR LEVEL, AND WHERE SHOWN ON THE DRAWINGS. MAXIMUM SPACE BETWEEN HORIZONTAL BOND BEAMS SHALL NOT EXCEED 8'-0".
11. UNLESS NOTED OTHERWISE ON PLANS, PROVIDE THE FOLLOWING ADDITIONAL VERTICAL REINFORCEMENT IN THE CELL IMMEDIATELY ADJACENT TO EACH SIDE OF A MASONRY OPENING AND IN THE CELL OF DISCONTINUOUS WALLS. THESE BARS ARE TO EXTEND FULL HEIGHT OF THE WALL OR IN THE CASE OF MASONRY OPENING AT MULTI-STORY WALLS, FROM STORY TO LEVEL ABOVE TO STORY LEVEL BELOW THE OPENING.
12. 6" AND 8"CMU WALLS – 1-#5
13. 10" AND 12" CMU WALLS – 2-#6
14. EXTEND ADDITIONAL REINFORCEMENT A MINIMUM OF 36 BAR DIAMETERS BEYOND THE OPENING.
15. THE MINIMUM LENGTH OF LAP FOR REINFORCING BARS EMBEDDED IN GROUT IS 48 BAR DIAMETERS, UNLESS SHOWN OTHERWISE ON THE DRAWINGS.
16. PLACE REINFORCING BARS BEFORE GROUTING. PLACE GROUT IN LIFTS NOT EXCEEDING 5 FEET. CONSOLIDATE EACH LIFT BY MECHANICAL VIBRATION. THE NEXT LIFT OF THE POUR MAY BE MADE AFTER THE INITIAL WATER LOSS AND RECONSOLIDATION OF THE PRIOR LIFT, WHILE IT IS STILL PLASTIC.
17. PROPERLY SECURE REINFORCING BARS TO MAINTAIN THE POSITIONS INDICATED ON THE DRAWINGS. BARS TO BE LOCATED IN CENTER OF CELLS UNLESS OTHERWISE NOTED.
18. ALL CMU SHALL BE BRACED DURING CONSTRUCTION FOR THE GOVERNING CODE LATERAL DESIGN LOADS UNTIL PERMANENT RESTRAINTS HAVE BEEN INSTALLED.
19. THE FOLLOWING STEPS ARE TO BE FOLLOWED WHEN LAYING MASONRY IN THE TEMPERATURES STATED BELOW:
    40 – 32 DEG F (MEAN DAILY AIR TEMPERATURE)
    HEAT MIXING WATER OR AGGREGATE TO 70° F.
    PROTECT MASONRY FROM RAIN OR SNOW FOR 24 HOURS.
    32 – 20 DEG F (MEAN DAILY AIR TEMPERATURE)
    HEAT MIXING WATER AND AGGREGATE TO 70° F.
    PROVIDE WIND BRACING FOR WIND VELOCITY IN EXCESS OF 15 M.P.H.
    COVER MASONRY WITH INSULATING BLANKETS FOR 24 HOURS AND PROVIDE HEAT SOURCES ON BOTH SIDES OF MASONRY CONSTRUCTION.
    BELOW 20° F (MEAN DAILY AIR TEMPERATURE)
    HEAT MIXING WATER & AGGREGATE TO 70° F.
    PROVIDE ENCLOSURES AND HEAT TO MAINTAIN 40° MINIMUM TEMPERATURE. TEMPERATURE OF MASONRY UNITS MUST BE 40° F MINIMUM WHEN LAID. MAINTAIN MASONRY ABOVE 40° F FOR 24 HOURS BY ENCLOSURES AND SUPPLEMENTAL HEAT.
6. INSPECTION AND TESTING OF MASONRY WORK WILL BE PERFORMED BY AN INDEPENDENT TESTING AGENCY, UNDER A SEPARATE CONTRACT WITH THE OWNER. IF THE MASONRY FAILS, CONTRACTOR SHALL PROMPTLY REPLACE MATERIALS OR REDO WORK WHICH HAS BEEN REJECTED BY ARCHITECT, ENGINEER AND/OR TESTING AGENCY, AT NO EXPENSE TO THE OWNER.

LATERAL FRAMING NOTES:

1. THE STRUCTURAL DESIGN OF THIS RESIDENCE WAS PERFORMED IN COMPLIANCE WITH THE INTERNATIONAL BUILDING AND MASSACHUSETTS STATE BUILDING CODE. FRAMING COMPONENTS AND FASTENERS AS IDENTIFIED IN THESE DRAWINGS AND NOTES ADEQUATELY RESIST THE LATERAL LOAD REQUIREMENTS AS DEFINED BY THE INTERNATIONAL RESIDENTIAL CODE FOR ONE AND TWO FAMILY DWELLINGS.

STATEMENT OF SPECIAL INSPECTIONS

VERIFICATION OF SOILS

CHECK IF REQUIRED	INSPECTION TASK (STANDARD & CODE REFERENCE)	CONTINUOUS INSPECTION	PERIODIC INSPECTION	SPECIAL INSPECTIONS FIRM	NOTES & SCOPE
☒	VERIFICATION OF SOILS 1705.6 & CHAPTER 18 IBC	☐	☒	TESTING LAB	
☒	VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	☐	☒	TESTING LAB	
☒	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	☐	☒	TESTING LAB	
☒	PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS	☐	☒	TESTING LAB	
☒	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL	☒	☐	TESTING LAB	
☒	PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERTY	☐	☒	TESTING LAB	

EXCAVATION AND FILLING

CHECK IF REQUIRED	INSPECTION TASK (STANDARD & CODE REFERENCE)	CONTINUOUS INSPECTION	PERIODIC INSPECTION	SPECIAL INSPECTIONS FIRM	NOTES & SCOPE
☒	VERIFY MATERIALS BELOW SHALLOW FOUNDATION ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	☐	☒	TESTING LAB	
☒	VERIFY EXCAVATION ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	☐	☒	TESTING LAB	
☒	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	☐	☒	TESTING LAB	
☒	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF COMPACTED MATERIAL	☒	☐	TESTING LAB	
☒	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	☐	☒	TESTING LAB	

CONCRETE CONSTRUCTION

CHECK IF REQUIRED	INSPECTION TASK (STANDARD & CODE REFERENCE)	CONTINUOUS INSPECTION	PERIODIC INSPECTION	SPECIAL INSPECTIONS FIRM	NOTES & SCOPE
☒	INSPECT REINFORCEMENT, INCLUDING PRETESTING TENDONS, AND VERIFY PLACEMENT. (ACI 318: 3.5, 7.1-7.7, IBC SECTION1913.4)	☐	☒	TESTING LAB	
☐	REINFORCING BAR WELDING: A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A 706; B. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM ⅝"; C. INSPECT ALL OTHER WELDS (IBC TABLE 1704.3, ITEM 5B. AWS: D1.4, ACI 318: 3.5.2)	☐	☐ ☒	TESTING LAB	
☐	INSPECT ANCHORS CAST IN CONCRETE. (IBC SECTION 1911.5)	☐	☒	TESTING LAB	
☒	INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS. B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.A	☒	☒	TESTING LAB	
☒	VERIFY USE OF REQUIRED DESIGN MIX. (ACI 318: CH.4, 5.2-5.4, 1904.2.2, IBC SECTION 1913.2, 1913.3)	☐	☒	TESTING LAB	
☒	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE. (ASTM C 172, ASTM C 21, ACI 318: 5.6, 5.8, IBC SECTION 1913.10)	☒	☐	TESTING LAB	
☐	INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES. (ACI 318: 5.9, 5.10, IBC SECTION 1913.9)	☐	☐	TESTING LAB	
☒	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES. (ACI 318: 5.11, 5.13, IBC SECTION 1913.9)	☐	☒	TESTING LAB	
☐	INSPECTION OF PRESTRESSED CONCRETE: A. APPLICATION OF PRESTRESSING FORCES; AND B. GROUTING OF BONDED PRESTRESSING TENDONS. (ACI 318: 18.20, ACI 318: 18.18.4)	☐	☐	TESTING LAB	
☐	INSPECT ERECTION OF PRECAST CONCRETE MEMBERS. (ACI 318: CH.16)	☐	☐	TESTING LAB	
☐	VERIFICATION OF IN-SITU CONCRETE, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE, AND PRIOR TO REMOVAL OF SHORES AND FORMS FORM BEAMS AND STRUCTURAL SLABS. (ACI 318: 6.2)	☐	☐	TESTING LAB	
☒	INSPECT FORM WORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED. (ACI 318: 6.1.1)	☐	☒	TESTING LAB	

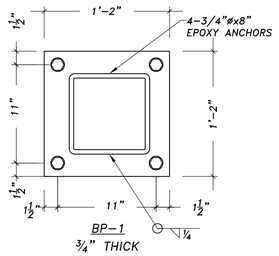
STRUCTURAL MASONRY (QUALITY ASSURANCE PROGRAM, LEVEL B)

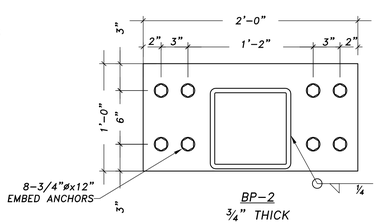
CHECK IF REQUIRED	INSPECTION TASK (STANDARD & CODE REFERENCE)	CONTINUOUS INSPECTION	PERIODIC INSPECTION	SPECIAL INSPECTIONS FIRM	NOTES & SCOPE
☒	VERIFICATION OF SLUMP FLOW AND VSI AS DELIVERED TO THE SIDE IN ACCORDANCE WITH ART. 1.5 B.1.b3			TESTING LAB	
☒	VERIFICATION OF F'M AND F'ACC PRIOR TO CONSTRUCTION, EXCEPT WHERE SPECIFICALLY EXEMPTED BY THE OCDE		☒		
☒	VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS				
☒	AS MASONRY CONSTRUCTION BEGINS, VERIFY THE FOLLOWING ARE INCOMPLIANCE. A. PROPORTIONS OF SITE-PREPARED MORTAR. B. CONSTRUCTION OF MORTAR JOINTS. C. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES. D. LOCATION OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES (ART. 2.6A, ART. 3.3B, ART. 3.4)	☐	☒	TESTING LAB	
☒	PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE: A. GROUT SPACE B. GRADE, TYPE AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS, AND ANCHORAGES. C. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES. D. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING TENDONS AND ANCHORAGES. E. CONSTRUCTION OF MORTAR JOINTS.SEC 2108.9.2.11, ITEM 2, SEC. 2104.3, 2104.4, ACI 318: SEC. 1.15.4, 2.1.2, SEC. 2.1.8.6.2, ACI 3.3G ART. 2.4, 3.4, ART. 1.8	☒	☒	TESTING LAB	
☒	VERIFY DURING CONSTRUCTION: A. SIZE AND LOCATION OF STRUCTURAL ELEMENTS B. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION. C. WELDING OF REINFORCEMENT. D. PREPARATION,CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMP, BELOW 40°F OR HOT WEATHER (TEMP ABOVE 90°) E. APPLICATION AND MEASUREMENT OF PRESTRSSING FORCE. F. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IN IN COMPLIANCE, SEC. 1.12, ART, 3.2D, ART, 3.4, ART, 2.6B, ART, 3.3B	☐	☒	TESTING LAB	
☒	GROUT PLACEMENT SHALL BE VERIFIED TO ENSURE COMPLIANCE WITH CODE AND CONSTRUCTION PROVISIONS, ART. 3.5	☒	☐	TESTING LAB	
☒	PREPARATION OF ANY REQUIRED GROUP SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS SHALL BE OBSERVED. SEC. 2105.3, 2105.4, 2105.5, ART.1.5	☐	☒	TESTING LAB	
☒	COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED. ART. 1.5	☐	☒	TESTING LAB	

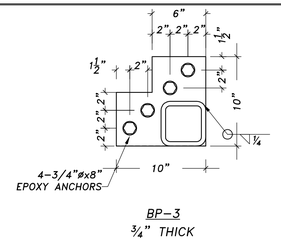
STRUCTURAL STEEL

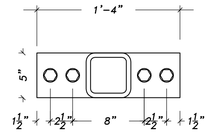
☒	SPECIAL INSPECTION AND NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IN BUILDINGS, STRUCTURES AND PORTIONS THEREOF SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360 CHAPTER N				

NOTE: QUALTIY CONTROL AS SPECIFIED IN AISC 360 SHALL BE PROVIDED BY THE FABRICATOR AND ERECTOR. QUALITY ASSURANCE AS SPECIFIED IN AISC 306 SHALL BE PROVIDED BY OTHERS



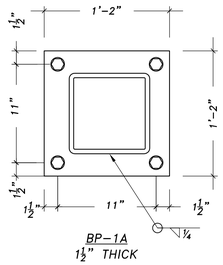


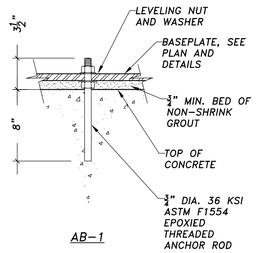


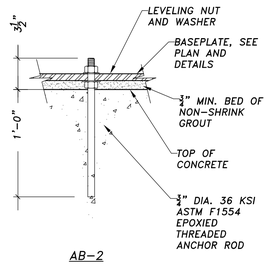


1 BASE PLATE DETAILS
Scale: 1 1/2" = 1'-0"

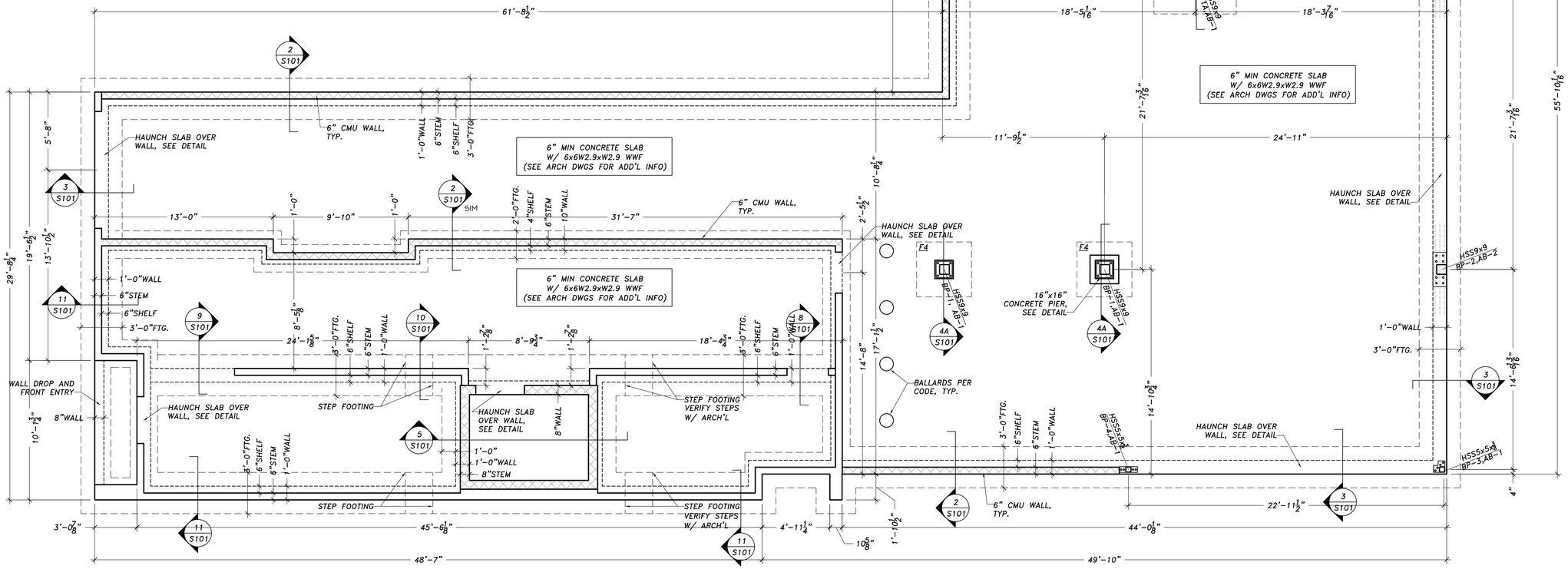
BASE PLATE NOTE:
ALL BASE PLATES SHALL BE ASTM A36 PLATE







2 ANCHOR BOLT DETAILS
Scale: 1 1/2" = 1'-0"



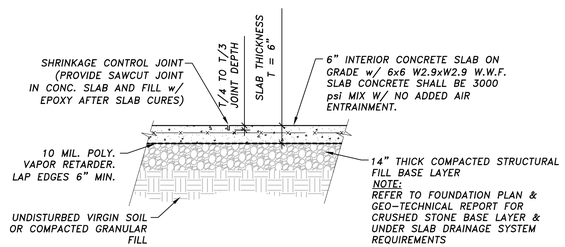
A FOUNDATION
1/4"=1'-0"

FOUNDATION NOTES:
1. *COORDINATE ALL FOUNDATION DIMENSIONS AND ELEVATIONS WITH SITE CONDITIONS AND ARCHITECTURAL DRAWINGS*

FOOTING SCHEDULE

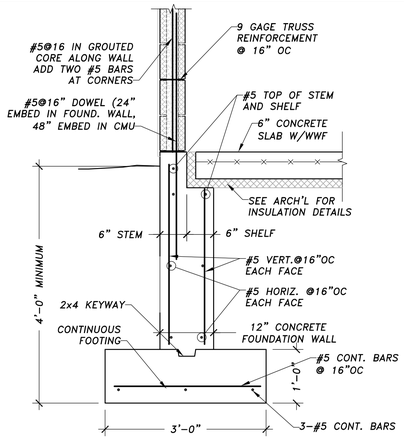
FOOTING ID TAG	FOOTING SIZE	REINFORCING
F4	4'-0"x4'-0"x12" THICK	(5) #5 E.W. BOT.
F6	6'-0"x6'-0"x16" THICK	(7) #6 E.W. BOT.

ALL DIMENSIONS, ELEVATIONS, SHELVES, BEAM POCKETS, CUT-OUTS, UNDERGROUND UTILITIES, PIERS, FOOTINGS, SLABS, AND ALL OTHER ITEMS SHALL BE FULLY COORDINATED WITH CIVIL, GEOTECHNICAL, MECHANICAL, ARCHITECTURAL AND ALL OTHER TRADES' DRAWINGS PRIOR TO CONSTRUCTION.

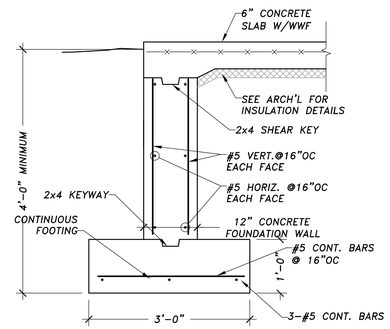


NOTES:
1. SAWCUT SHRINKAGE CONTROL JOINTS SHALL BE PROVIDED WITHIN 12 HRS. OF SLAB PLACEMENT, AS SOON AS CONCRETE IS CAPABLE OF SUPPORTING SAWCUTTING EQUIPMENT.
2. LOCATE SHRINKAGE CONTROL JOINTS AS INDICATED ON SLAB PLAN, OR AT MAXIMUM SPACING OF 20 FT. O.C. IF NOT INDICATED ON PLANS. RESULTING SHAPE SHALL NOT BE GREATER THAN 400 S.F. NOR EXCEED A 1.5 : 1 LENGTH TO WIDTH RATIO.
3. OPTIONALLY, THE SHRINKAGE CONTROL MAY SUBSTITUTE PRE-FABRICATED PLASTIC STRIPS INSTEAD OF SAWCUTTING. SUBMIT CATALOG CUTS FOR APPROVAL PRIOR TO USING.
4. INSTALLATION OF ALL NON-STRUCTURAL CONCRETE SLABS-ON-GRADE SHALL CONFORM TO ALL REQUIREMENTS OF THE LATEST ADDITIONS OF BOTH, ACI-360 AND ACI-302.

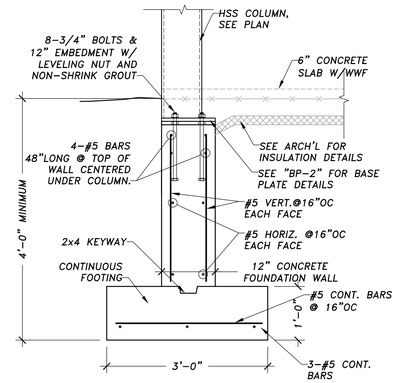
1 SLAB DETAIL
Scale: 3/4" = 1'-0"



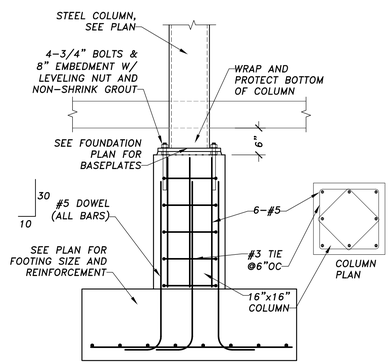
2 FROSTWALL DETAIL
Scale: 3/4" = 1'-0"



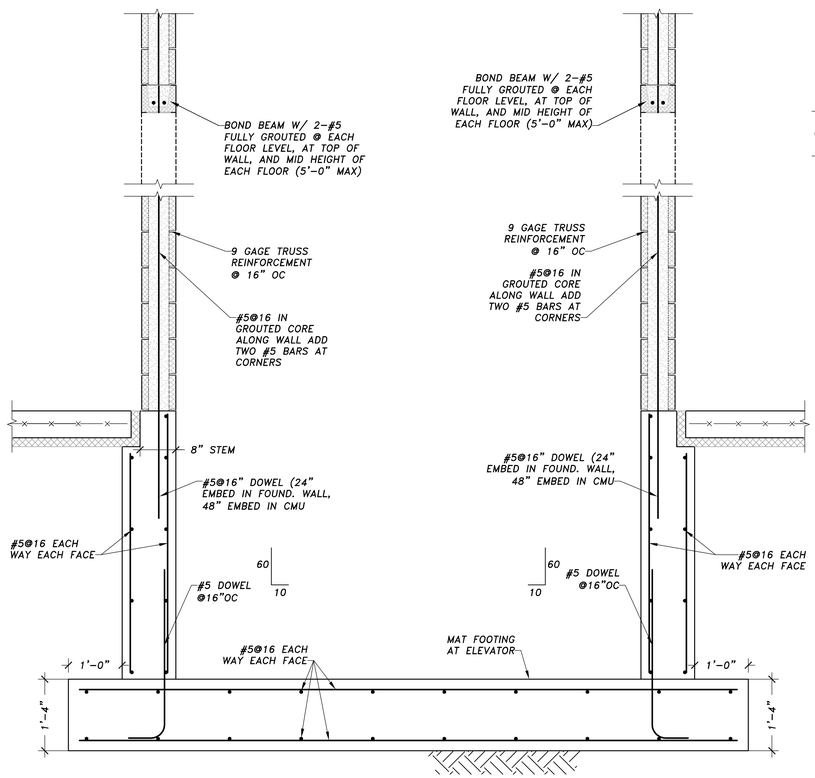
3 SECTION THROUGH WALL DROP
Scale: 3/4" = 1'-0"



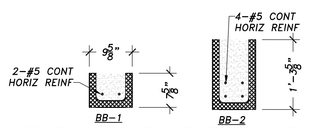
4 SECTION THROUGH STEEL COLUMN
Scale: 3/4" = 1'-0"



4A SECTION THROUGH CONC. PIER
Scale: 3/4" = 1'-0"



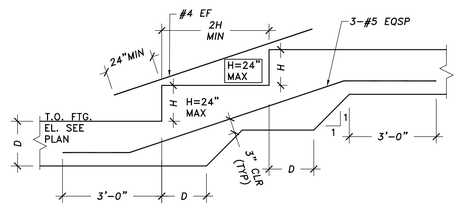
5 ELEVATOR SECTION
Scale: 3/4" = 1'-0"



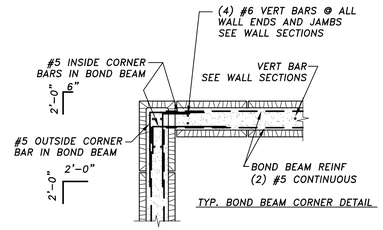
PROVIDE BB-1 BOND BEAMS AT OPENINGS SMALLER THAN 6'-0". PROVIDE BB-2 AT OPENINGS BETWEEN 6'-0 AND 10'-0"

NOTES:
1. BOND BEAMS & LINTELS SHALL BE FULLY GROUTED
2. THESE BOND BEAMS APPLY TO WALLS WHERE BOND BEAMS ARE NOT SPECIFICALLY REFERENCED
3. LINTELS SHALL BE CONSTRUCTED USING SOLID BOTTOM MASONRY UNITS
4. GROUTED LINTELS SHALL EXTEND 8" BEYOND THE FACE OF OPENINGS ON EACH END

6 BOND BEAM DETAILS
Scale: 3/4" = 1'-0"



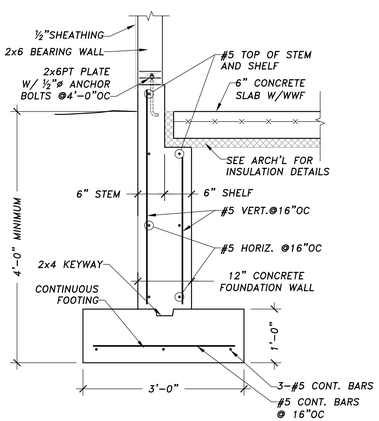
7 SECTION THROUGH WALL DROP
Scale: 3/4" = 1'-0"



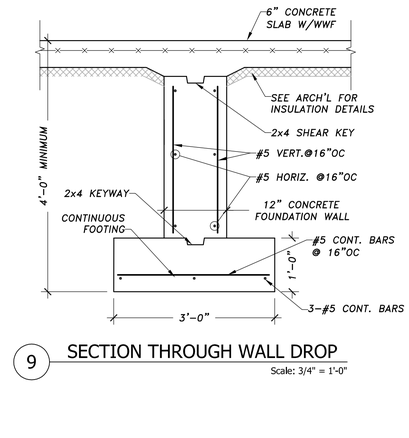
TYP. BOND BEAM CORNER DETAIL

1. PROVIDE CORNER BARS TO MATCH AND LAP WITH BOND BEAM REINF OR PROVIDE 90° HOOK.
2. EXTEND HORIZ JOINT REINF TO END OF WALL IN BOTH DIRECTIONS SO THAT IT LAPS AT THE CORNER.

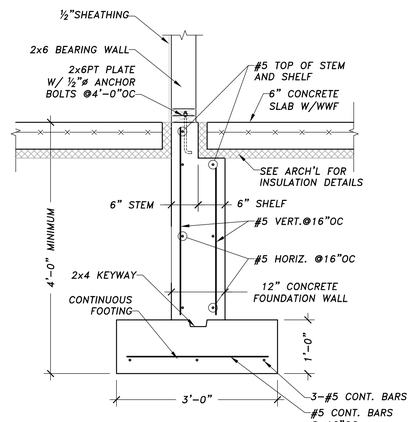
8 TYP. C.M.U CORNERS
Scale: 3/4" = 1'-0"



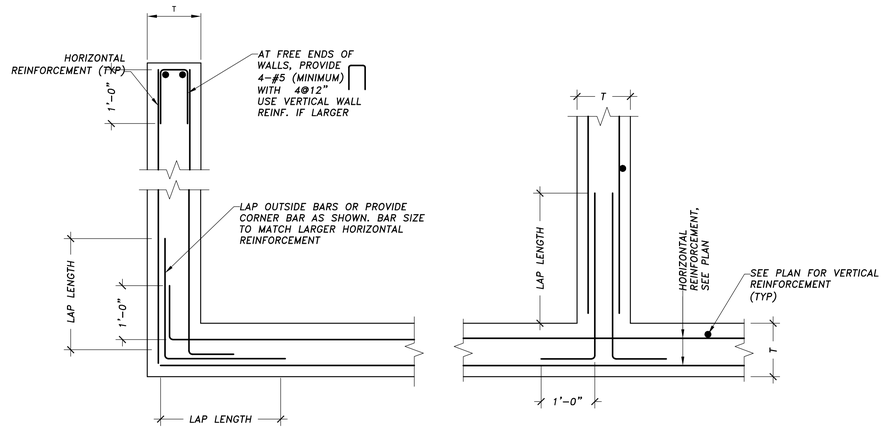
11 FROSTWALL DETAIL
Scale: 3/4" = 1'-0"



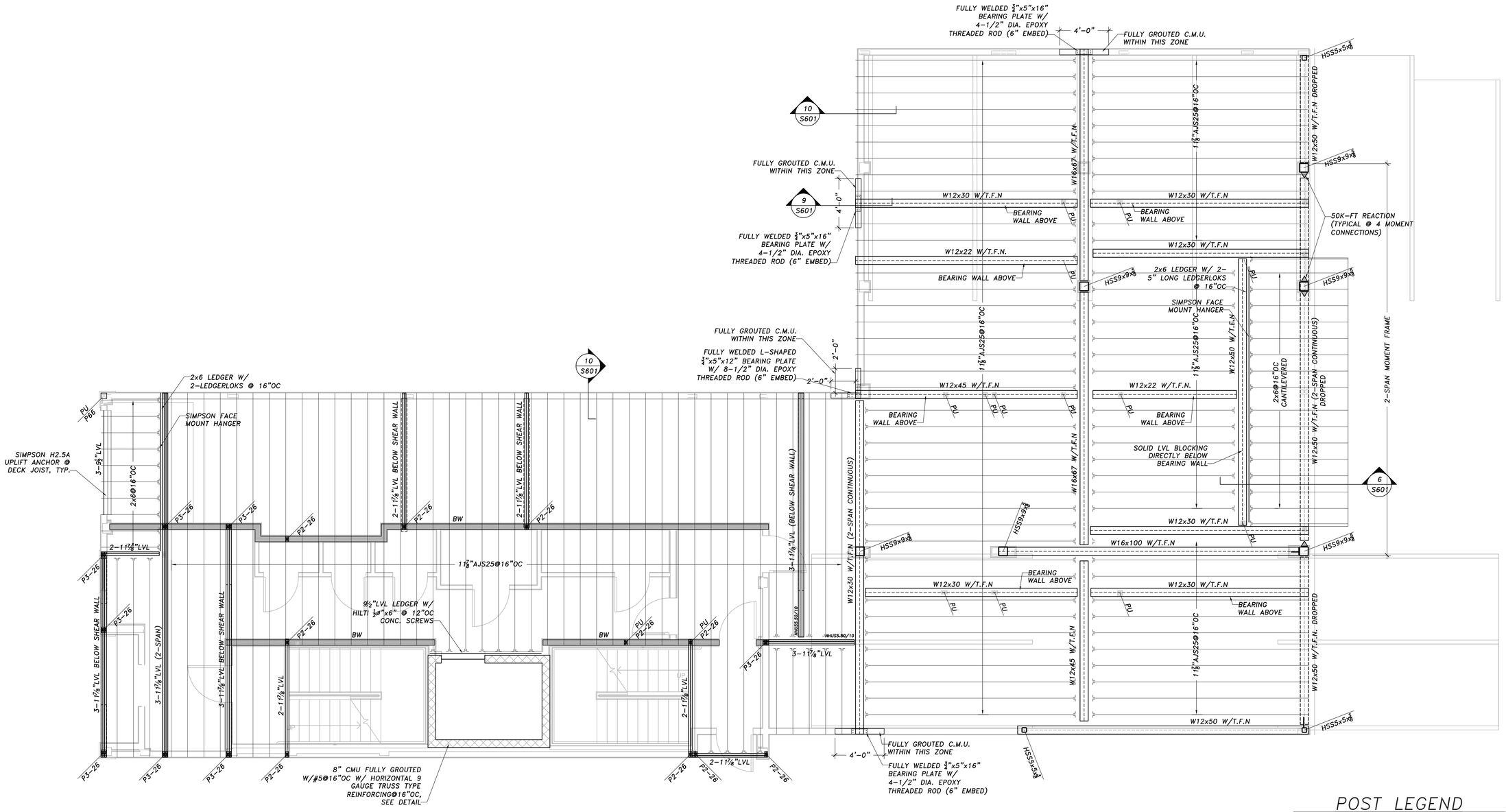
9 SECTION THROUGH WALL DROP
Scale: 3/4" = 1'-0"



10 FROSTWALL DETAIL
Scale: 3/4" = 1'-0"



12 TYPICAL HORIZONTAL WALL REINFORCEMENT
Scale: 3/4" = 1'-0"



A SECOND FLOOR FRAMING PLAN

1/4"=1'-0"

NOTES:

1. ALL INDIVIDUAL LVLS ARE 1¾" THICK UNLESS NOTED OTHERWISE ON PLAN.
2. HEADERS ARE AS FOLLOWS UNLESS NOTED OTHERWISE:

   3-2x8 — J1-26

3. BEAMS COMPRISED OF 3 LVLS OR MORE SHALL BE BOLTED TOGETHER WITH A MINIMUM OF 2-1/2"ø BOLTS AT 16" ON CENTER OR 3-¼"ø DIAMETER SELF TAPPING LAG SCREWS AT 16" ON CENTER, ALTERNATING INSERTION SIDES, FOLLOW MANUF. SPECS, UNLESS NOTED OTHERWISE ON DRAWING.
4. BW DENOTES BEARING WALLS CONSISTING OF 2x6@16"OC. SEE FRAMING NOTES FOR HORIZ. BRACING.
5. = SHEAR WALL ABOVE, SEE LATERAL PLAN

POST LEGEND

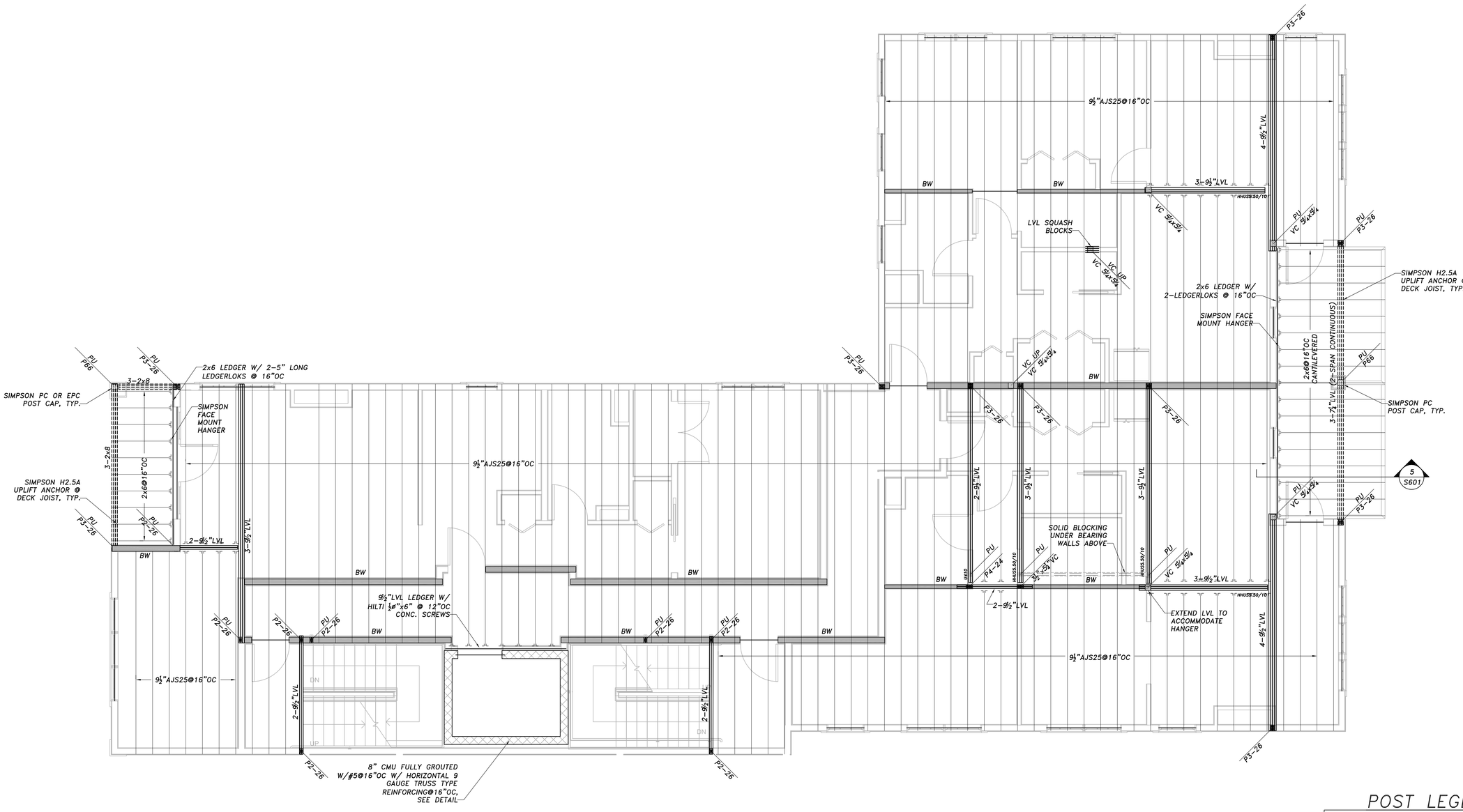
(PU) POST ABOVE FRAMING LEVEL
POST BELOW FRAMING LEVEL

NUMBER OF STUDS IF APPLICABLE

P3-26

SIZE OF STUD OR DIMENSION OF SOLID POST

TYPE OF POST:
P-POST, J-JACK, VC-VERSA COLUMN, LC-LALLY COLUMN, HSS-HOLLOW STRUCTURAL SECTION



A THIRD FLOOR FRAMING PLAN

1/4"=1'-0"

NOTES:

1. ALL INDIVIDUAL LVLS ARE 1¾" THICK UNLESS NOTED OTHERWISE ON PLAN.
2. HEADERS ARE AS FOLLOWS UNLESS NOTED OTHERWISE:

   3-2x10 J1-26

3. BEAMS COMPRISED OF 3 LVLS OR MORE SHALL BE BOLTED TOGETHER WITH A MINIMUM OF 2-1/2"ø BOLTS AT 16" ON CENTER OR 3-½"ø DIAMETER SELF TAPPING LAG SCREWS AT 16" ON CENTER, ALTERNATING INSERTION SIDES, FOLLOW MANUF. SPECS, UNLESS NOTED OTHERWISE ON DRAWING.
4. BW DENOTES BEARING WALLS CONSISTING OF 2x6@16"OC. SEE FRAMING NOTES FOR HORIZ. BRACING.

POST LEGEND

(PU) POST ABOVE FRAMING LEVEL

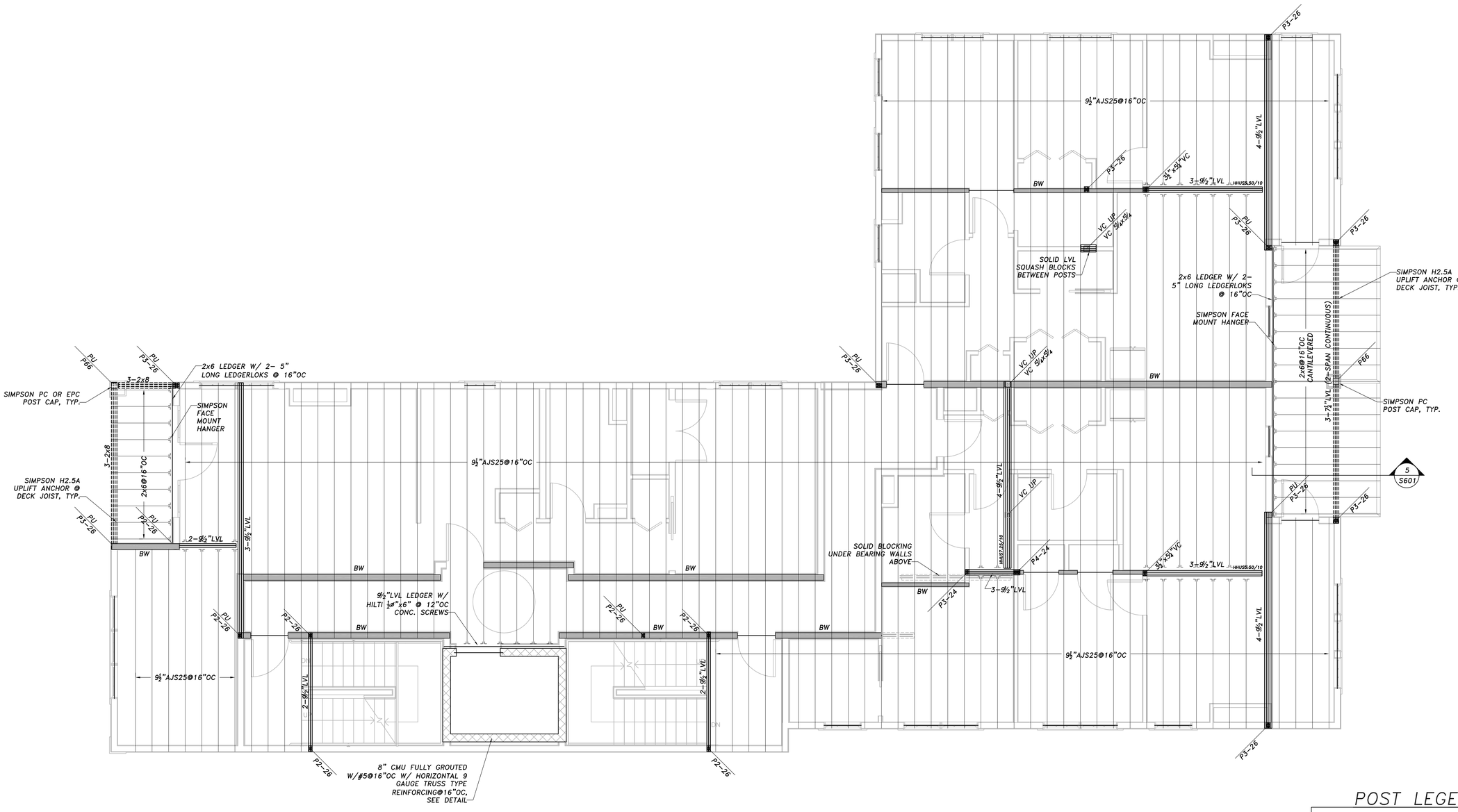
POST BELOW FRAMING LEVEL

NUMBER OF STUDS IF APPLICABLE

P3-26

SIZE OF STUD OR DIMENSION OF SOLID POST

TYPE OF POST:
P-POST,J-JACK,VC-VERSA COLUMN, LC-LALLY COLUMN, HSS-HOLLOW STRUCTURAL SECTION



A FOURTH FLOOR FRAMING PLAN

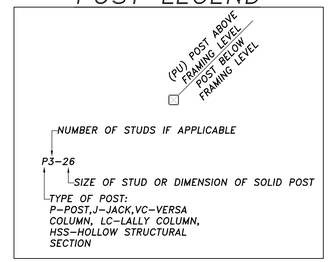
1/4"=1'-0"

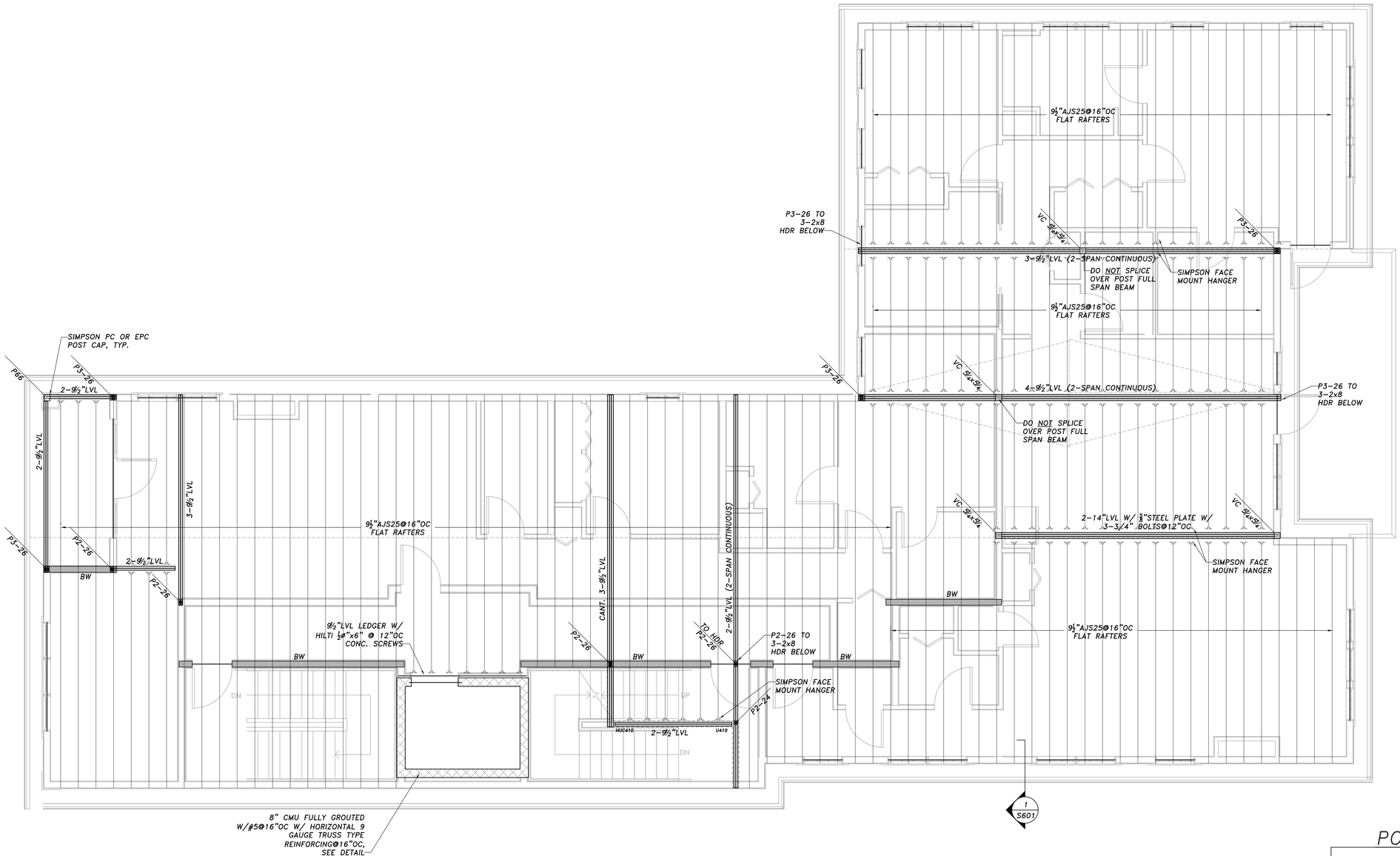
NOTES:

1. ALL INDIVIDUAL LVLS ARE 1¾" THICK UNLESS NOTED OTHERWISE ON PLAN.
2. HEADERS ARE AS FOLLOWS UNLESS NOTED OTHERWISE:

3. BEAMS COMPRISED OF 3 LVLS OR MORE SHALL BE BOLTED TOGETHER WITH A MINIMUM OF 2-1/2"ø BOLTS AT 16" ON CENTER OR 3-½"ø DIAMETER SELF TAPPING LAG SCREWS AT 16" ON CENTER, ALTERNATING INSERTION SIDES, FOLLOW MANUF. SPECS, UNLESS NOTED OTHERWISE ON DRAWING.
4. BW DENOTES BEARING WALLS CONSISTING OF 2x6@16"OC. SEE FRAMING NOTES FOR HORIZ. BRACING.

POST LEGEND





A ROOF FRAMING PLAN

1/4"=1'-0"

NOTES:

1. ALL INDIVIDUAL LVLS ARE 1¾" THICK UNLESS NOTED OTHERWISE ON PLAN.
2. HEADERS ARE AS FOLLOWS UNLESS NOTED OTHERWISE:

3-2x8 J1-26

3. BEAMS COMPRISED OF 3 LVLS OR MORE SHALL BE BOLTED TOGETHER WITH A MINIMUM OF 2-1/2"ø BOLTS AT 16" ON CENTER OR 3-¼"ø DIAMETER SELF TAPPING LAG SCREWS AT 16" ON CENTER, ALTERNATING INSERTION SIDES, FOLLOW MANUF. SPECS, UNLESS NOTED OTHERWISE ON DRAWING.
4. BW DENOTES BEARING WALLS CONSISTING OF 2x6@16"OC. SEE FRAMING NOTES FOR HORIZ. BRACING.
5. PROVIDE SIMPSON H8 UPLIFT ANCHOR AT EACH JOIST, SEE DETAILS

POST LEGEND

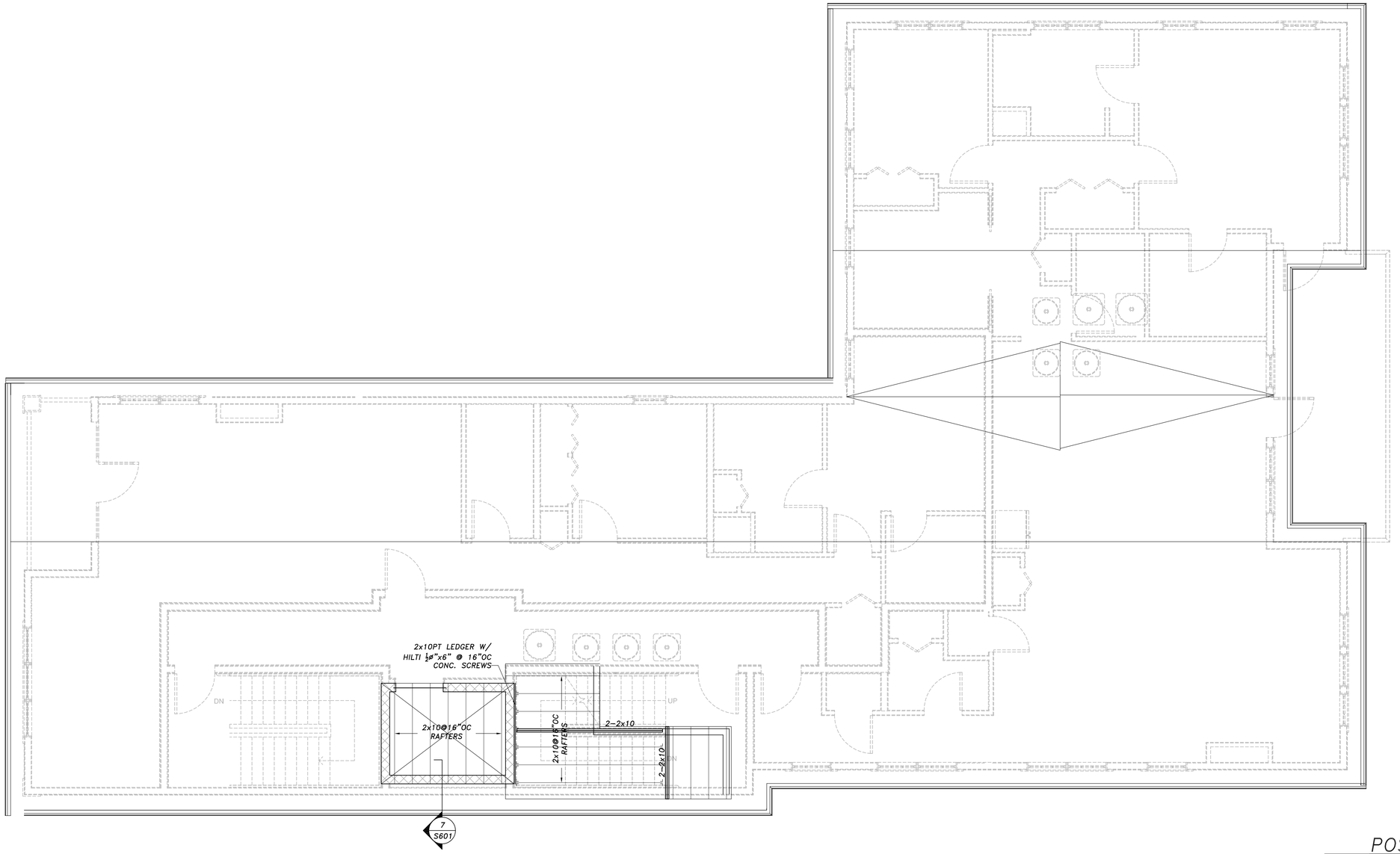
(PU) POST ABOVE FRAMING LEVEL
□ POST BELOW FRAMING LEVEL

NUMBER OF STUDS IF APPLICABLE

P3-26

SIZE OF STUD OR DIMENSION OF SOLID POST

TYPE OF POST:
P-POST, J-JACK, VC-VERSA COLUMN, LC-LALLY COLUMN, HSS-HOLLOW STRUCTURAL SECTION

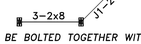


A HEAD HOUSE FRAMING

1/4"=1'-0"

NOTES:

1. ALL INDIVIDUAL LVLS ARE 1¾" THICK UNLESS NOTED OTHERWISE ON PLAN.
2. HEADERS ARE AS FOLLOWS UNLESS NOTED OTHERWISE:



3. BEAMS COMPRISED OF 3 LVLS OR MORE SHALL BE BOLTED TOGETHER WITH A MINIMUM OF 2-1/2"ø BOLTS AT 16" ON CENTER OR 3-¼"ø DIAMETER SELF TAPPING LAG SCREWS AT 16" ON CENTER, ALTERNATING INSERTION SIDES, FOLLOW MANUF. SPECS, UNLESS NOTED OTHERWISE ON DRAWING.
4. BW DENOTES BEARING WALLS CONSISTING OF 2x6@16"OC. SEE FRAMING NOTES FOR HORIZ. BRACING.

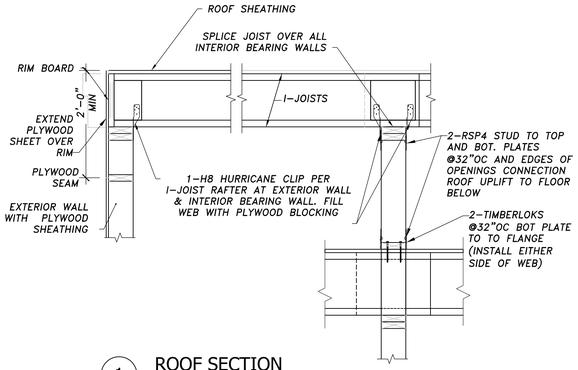
POST LEGEND

NUMBER OF STUDS IF APPLICABLE

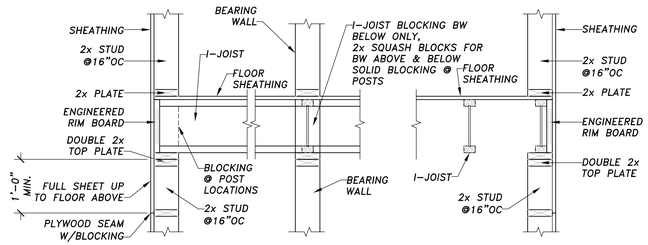
P3-26

SIZE OF STUD OR DIMENSION OF SOLID POST

TYPE OF POST:
P-POST,J-JACK,VC-VERSA COLUMN, LC-LALLY COLUMN, HSS-HOLLOW STRUCTURAL SECTION

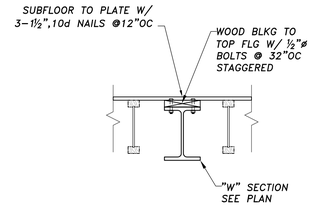


1 ROOF SECTION
Scale: 3/4" = 1'-0"

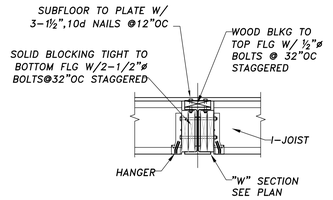


2 TYPICAL I-JOIST FLOOR DETAILS
Scale: 3/4" = 1'-0"

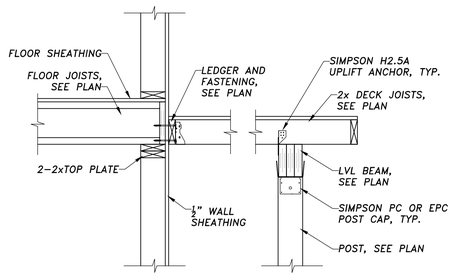
NOTE:
REFER TO MNFR SPECS FOR ADD'L INFORMATION.



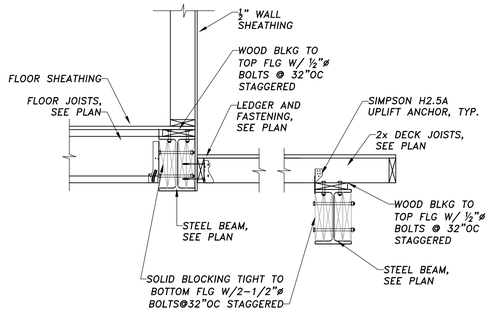
3 TYPICAL STEEL BEAM
Scale: 3/4" = 1'-0"



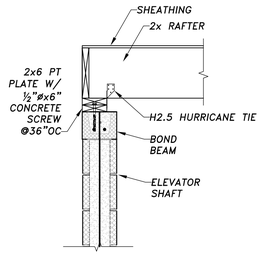
4 TYPICAL STEEL BEAM FLUSH
Scale: 3/4" = 1'-0"



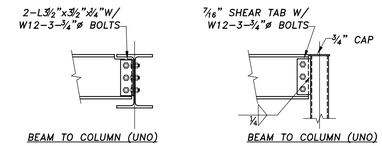
5 TYPICAL SECTION THROUGH PORCH
Scale: 3/4" = 1'-0"



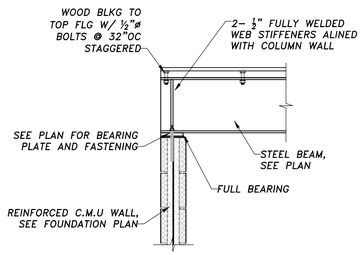
6 SECTION THROUGH LOW PORCH
Scale: 3/4" = 1'-0"



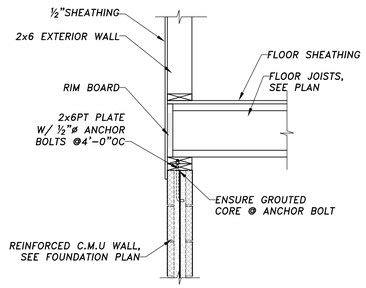
7 ROOF AT ELEVATOR
Scale: 3/4" = 1'-0"



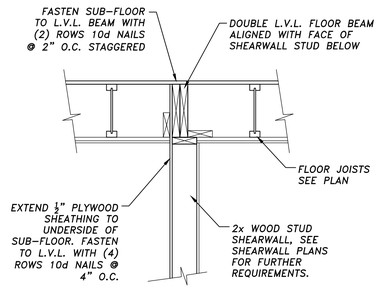
8 TYPICAL BEAM TO COL. CONNECTIONS
Scale: 3/4"=1'-0"



9 SECTION THRU STEEL BEAM BEARING
Scale: 3/4" = 1'-0"

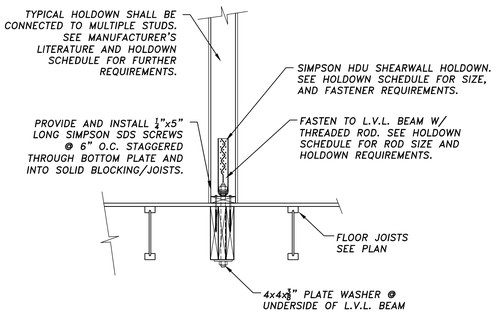


10 TYPICAL JOIST BEARING
Scale: 3/4" = 1'-0"



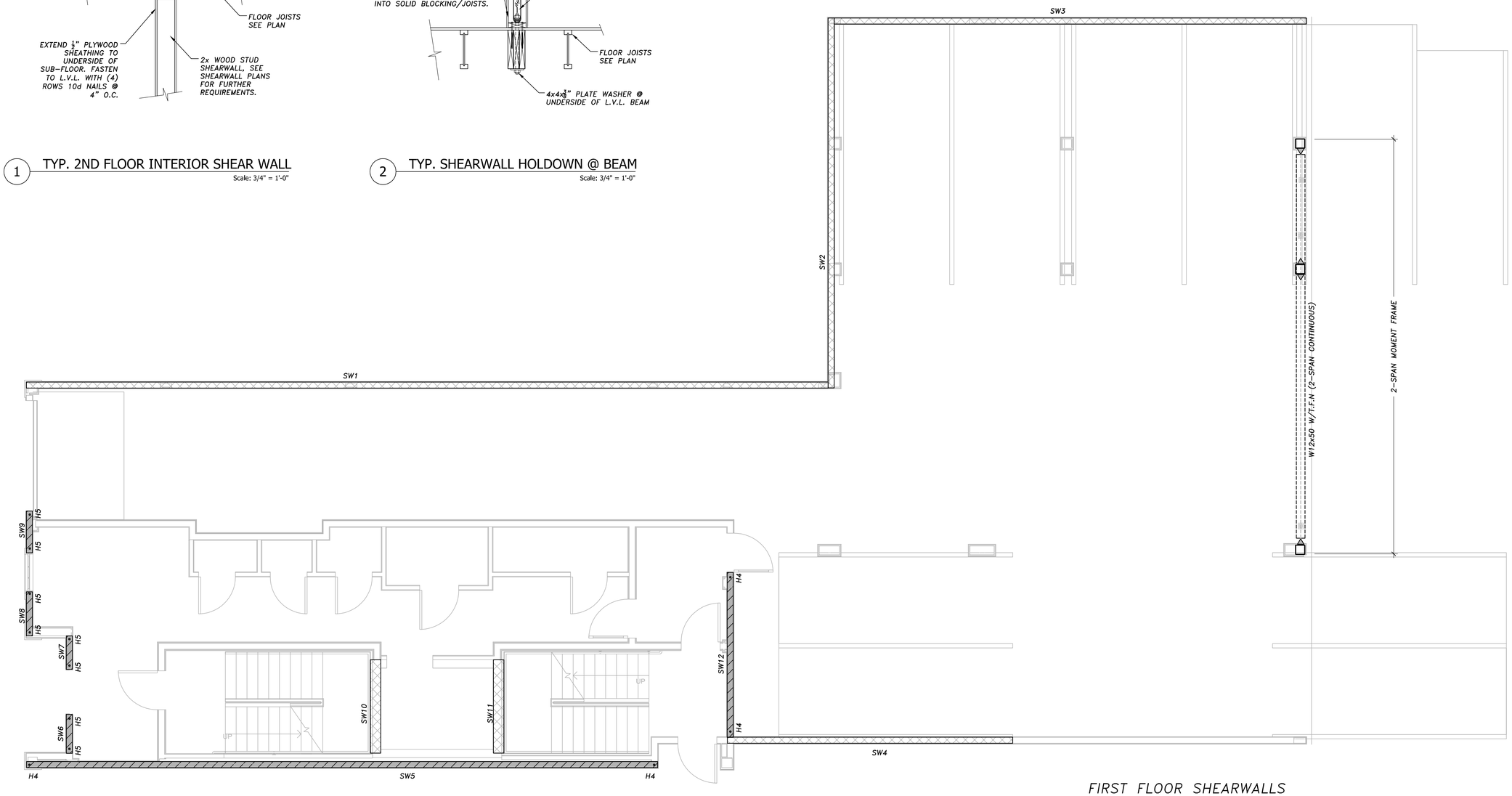
1 TYP. 2ND FLOOR INTERIOR SHEAR WALL

Scale: 3/4" = 1'-0"



2 TYP. SHEARWALL HOLDOWN @ BEAM

Scale: 3/4" = 1'-0"



A FIRST FLOOR SHEAR WALLS

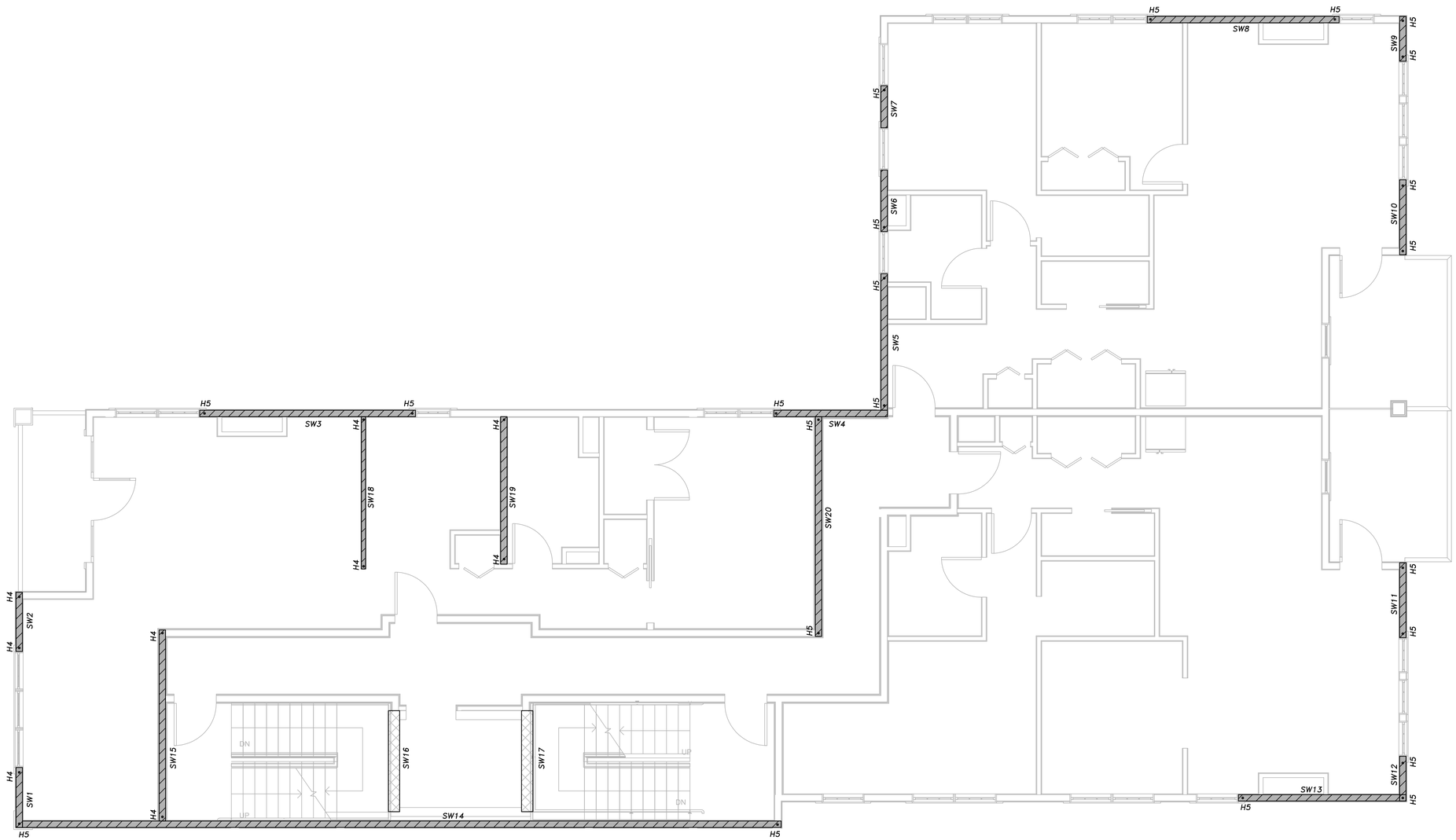
1/4"=1'-0"

FIRST FLOOR SHEARWALLS

SHEARWALL ID TAG	PLYWOOD FACE	SHEETROCK FACE	WALL STUDS @ PANEL EDGES	BLOCKING @ EDGES?	FASTENING @ PANEL EDGES
SW1	C.M.U. SHEAR WALL. SEE ELEVATOR DETAILS				
SW2	C.M.U. SHEAR WALL. SEE ELEVATOR DETAILS				
SW3	C.M.U. SHEAR WALL. SEE ELEVATOR DETAILS				
SW4	C.M.U. SHEAR WALL. SEE ELEVATOR DETAILS				
SW5	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 4" O.C.
SW6	BOTH	INNER	SINGLE 2x6	BLOCKED	8d @ 3" O.C.
SW7	BOTH	INNER	SINGLE 2x6	BLOCKED	8d @ 3" O.C.
SW8	BOTH	INNER	SINGLE 2x6	BLOCKED	8d @ 3" O.C.
SW9	BOTH	INNER	SINGLE 2x6	BLOCKED	8d @ 3" O.C.
SW10	C.M.U. SHEAR WALL. SEE ELEVATOR DETAILS				
SW11	C.M.U. SHEAR WALL. SEE ELEVATOR DETAILS				
SW12	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 4" O.C.

HOLDOWN SCHEDULE

HOLDOWN ID TAG ON PLAN	SIMPSON MODEL #	HOLDOWN FASTENED TO:	FASTENERS TO FRAMING	THREADED ROD DIAMETER	THREADED ROD EMBEDMENT
H4	HDU4-SDS2.5	DOUBLE WALL STUD	SDS SCREWS	5/8" DIA.	12"
H5	HDU5-SDS2.5	DOUBLE WALL STUD	SDS SCREWS	5/8" DIA.	12"



A SECOND FLOOR SHEAR WALLS

1/4"=1'-0"

HOLDOWN SCHEDULE

HOLDOWN ID TAG ON PLAN	SIMPSON MODEL #	HOLDOWN FASTENED TO:	FASTENERS TO FRAMING	THREADED ROD DIAMETER	THREADED ROD EMBEDMENT
H4	HDU4-SDS2.5	DOUBLE WALL STUD	SDS SCREWS	5/8" DIA.	12"
H5	HDU5-SDS2.5	DOUBLE WALL STUD	SDS SCREWS	5/8" DIA.	12"

SECOND FLOOR SHEARWALLS

SHEARWALL ID TAG	PLYWOOD FACE	SHEETROCK FACE	WALL STUDS @ PANEL EDGES	BLOCKING @ EDGES?	FASTENING @ PANEL EDGES
SW1	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 6" O.C.
SW2	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 6" O.C.
SW3	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 4" O.C.
SW4	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 4" O.C.
SW5	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 4" O.C.
SW6	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 4" O.C.
SW7	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 4" O.C.
SW8	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 6" O.C.
SW9	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 4" O.C.
SW10	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 4" O.C.
SW11	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 4" O.C.
SW12	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 4" O.C.
SW13	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 6" O.C.
SW14	OUTER	INNER	SINGLE 2x6	BLOCKED	8d @ 6" O.C.
SW15	EITHER	BOTH	SINGLE 2x6	BLOCKED	8d @ 4" O.C.
SW16	C.M.U. SHEAR WALL. SEE ELEVATOR DETAILS				
SW17	C.M.U. SHEAR WALL. SEE ELEVATOR DETAILS				
SW18	EITHER	BOTH	SINGLE 2x4	BLOCKED	8d @ 6" O.C.
SW19	EITHER	BOTH	SINGLE 2x4	BLOCKED	8d @ 6" O.C.
SW20	EITHER	BOTH	SINGLE 2x4	BLOCKED	8d @ 4" O.C.